D: B-3922

CONTENTS: -L-

ROJECT: 33356.1.1

### STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

**DIVISION OF HIGHWAYS** 

GEOTECHNICAL ENGINEERING UNIT

### STRUCTURE SUBSURFACE INVESTIGATION

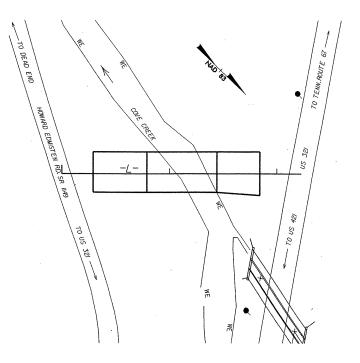
STATE PROJECT33356.I.I I.D. NO. B-3922

F.A. PROJECT

COUNTY WATAUGA

DESCRIPTION BRIDGE NO. 316 ON SR-1149 OVER

COVE CREEK



NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

				CONS	Τ.
				P.E.	
STATE	PROJ. NO.	F. A. PROJ. NO.		DESCRIP	TION
N.C.		B-3922		1	17
STATE	STATE PR	OJECT REFERENCE	NO.	SHEET NO.	TOTAL SHEETS

#### CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT & (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

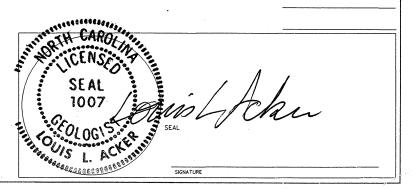
THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HINSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT, THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

INVESTIGATED BY L.L. ACKER PERSONNEL L.L. ACKER

CHECKED BY W.D. FRYE D.O. CHEEK

SUBMITTED BY W.D. FRYE C.J. COFFEY

DATE JUNE 2004 C.K. ROSE



DRAWN BY: PO LOCKAMY

#### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

#### DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

### SUBSURFACE INVESTIGATION

					SOIL AND RO	CK LEGEND, TERM	IS, SYMBOLS,	, AND ABBI	REVIATIONS		
	SOIL	DESCRIPTION			GRADATION			F	ROCK DESCRIPTION		TERMS AND DEFINITIONS
SOIL IS CONSI	DERED TO BE THE UNCONSOLIDATED	D, SEMI-CONSOLIDATED OR WEA	ATHERED EARTH MATERIALS	WELL GRADED- INDICATES A UNIFORM- INDICATES THAT S	GOOD REPRESENTATION OF PARTICLE SIZES F OIL PARTICLES ARE ALL APPROXIMATELY THE	ROM FINE TO COARSE SAME SIZE.(ALSO			RIAL THAT WHEN TESTED,WOULD YIELD ICH NON-COASTAL PLAIN MATERIAL WOUL		ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER.
	PENETRATED WITH A CONTINUOUS R FOOT ACCORDING TO STANDARD F			POORLY GRADED)	IXTURE OF UNIFORM PARTICLES OF TWO OR M		SPT REFUSAL IS P	PENETRATION BY A SPLI	IT SPOON SAMPLER EQUAL TO OR LESS FRANSITION BETWEEN SOIL AND ROCK IS	HAN 0.1 FOOT PER 60 BLOWS.	AQUIFER - A WATER BEARING FORMATION OR STRATA.
CLASSIFICATIO	N IS BASED ON THE AASHTO SYSTE	EM AND BASIC DESCRIPTIONS	GENERALLY SHALL INCLUDE:	The state of the s	ANGULARITY OF GRAINS	0.12 0.120.	OF WEATHERED RO	CK.		UFIEN REPRESENTED BY A ZUNE	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
	ICAL COMPOSITION, ANGULARITY, ST	RUCTURE, PLASTICITY, ETC. EXA	MPLE:		ESS OF SOIL GRAINS ARE DESIGNATED BY TH	TERMS; ANGULAR,		ARE TYPICALLY DIVIDED			ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
		TH INTERBEDOED FINE SAND LAYERS,HIGHL		SUBANGULAR, SUBROUNDED, 0			WEATHERED ROCK (WR)	PER FO	DASTAL PLAIN MATERIAL THAT YIELDS S DOT.	T N VALUES > 100 BLOWS	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL
		) AASHTO CLASSIF	ICATION	MINEDAL NAMES CHELL AS ON	MINERALOGICAL COMPOSITION AND AND AND AND AND AND AND AND AND AN		CRYSTALLINE		O COARSE GRAIN IGNEOUS AND METAMOR		AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
GENERAL CLASS.	GRANULAR MATERIALS (≦5% PASSING #200)	SILT-CLAY MATERIALS (>85% PASSING #200)	ORGANIC MATERIALS	WHENEVER THEY ARE CONSIDE	ARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE ERED OF SIGNIFICANCE.	used in Descriptions	ROCK (CR)	GNEISS,	YIELD SPT REFUSAL IF TESTED, ROCK , GABBRO, SCHIST, ETC.		CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP	A-I A-3 A-2	A-4 A-5 A-6 A-			COMPRESSIBILITY		NON-CRYSTALLINE		O COARSE GRAIN METAMORPHIC AND NON NTARY ROCK THAT WOULD YEILD SPT RE		COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
CLASS. A-	1-a A-1-b A-2-4 A-2-5 A-2-6	6 A-2-7 A-7-	-S A-3 A-6, A-7	SLIGHTLY COMPRES		LESS THAN 30	ROCK (NCR)  COASTAL PLAIN	INCLUDE	ES PHYLLITE, SLATE, SANDSTONE, ETC.		OF SLOPE.
SYMBOL 800	00000000			MODERATELY COMP HIGHLY COMPRESSI		GREATER THAN 50	SEDIMENTARY ROCK	SPT REI	IL PLAIN SEDIMENTS CEMENTED INTO RO FUSAL. ROCK TYPE INCLUDES LIMESTONE		CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
% PASSING			SILT-	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	PERCENTAGE OF MATERIA	L	(CP)	SHELL I	BEDS, ETC. WEATHERING		DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
# 10 50 # 40 30	MX 50 MX 51 MN		GRANULAR CLAY PEA		GRANULAR SILT- CLAY SOILS SOILS	OTHER MATERIAL	SDEAN BOOK S	TOTAL COVETAL O DOTAL			ROCKS OR CUTS MASSIVE ROCK.
<b>#</b> 200 15	MX 25 MX 10 MX 35 MX 35 MX 35 MX	X35 MX36 MN 36 MN 36 MN 36 I	MN SUILS	TRACE OF ORGANIC MATTER LITTLE ORGANIC MATTER		ACE 1 - 10%		R IF CRYSTALLINE.	T, FEW JOINTS MAY SHOW SLIGHT STAIN	ING. RUCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
LIQUIÓ LIMIT		X41 MN 40 MX 41 MN 40 MX 41 M		MODERATELY ORGANIC		TTLE 10 - 20% ME 20 - 35%			TS STAINED, SOME JOINTS MAY SHOW TH		DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF
ļ		11 MN 10 MX 10 MX 11 MN 11 M	HIGH			GHLY 35% AND ABOVE		ALS ON A BROKEN SPEC CRYSTALLINE NATURE.	CIMEN FACE SHINE BRIGHTLY, ROCK RINGS	UNDER HAMMER BLOWS IF	THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX		MX 8 MX 12 MX 16 MX No	AMOUNTS OF SOIL		GROUND WATER		SLIGHT ROCK (	GENERALLY FRESH, JOIN	ITS STAINED AND DISCOLORATION EXTEND		FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRA	AVEL AND SAND GRAVEL AND S			WATER WATER	LEVEL IN BORE HOLE IMMEDIATELY AFTER	DRILLING.			NTAIN CLAY. IN GRANITOID ROCKS SOME COLORED. CRYSTALLINE ROCKS RING UND		FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
	SAND SAND GRAVEE AND S	30123 30123			WATER LEVEL AFTER 24 HOURS.	•	1		CK SHOW DISCOLORATION AND WEATHERI		FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING AS A	EXCELLENT TO GOOD	FAIR TO POOR	FAIR TO POOR UNSUIT	BLE PERCHE	D WATER, SATURATED ZONE OR WATER BEAM	RING STRATA			SPARS ARE DULL AND DISCOLORED, SOME BLOWS AND SHOWS SIGNIFICANT LOSS OF		PARENT MATERIAL.
SUBGRADE	DIOC A 7 E - 1 1	- 30 : P.I. OF A-7-6 > L		SPRING	OR SEEPAGE			RESH ROCK.	seems that shows storm terms 2000 or	STRENGTH NO CONTINUED	FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
		NCY OR DENSENES			MISCELLANEOUS SYMBOL	5			SCOLORED OR STAINED, IN GRANITOID RO RITY SHOW KAOLINIZATION, ROCK SHOWS		FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN
	COMPACTNESS OF	RANGE OF STANDARD	RANGE OF UNCONFINED	П ROADWAY EMBAN	SPT CPT		(MOD. SEV.) AND CA	AN BE EXCAVATED WITH	A GEOLOGIST'S PICK. ROCK GIVES *CLU		THE FIELD.
PRIMARY SO	CONSISTENCY	PENETRATION RESISTENCE (N-VALUE)	COMPRESSIVE STRENGTH (TONS/FT <sup>2</sup> )	WITH SOIL DESC		NG SAMPLE DESIGNATIONS		TED, WOULD YIELD SPT		540 AUG EWOENT DUT DEDUCE	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
GENERAL	VERY LOOSE	<4		SOIL SYMBOL	AUGER BORING				DISCOLORED OR STAINED.ROCK FABRIC C L. IN GRANITOID ROCKS ALL FELDSPARS		LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
GRANULA	R LUUSE	4 TO 10 10 TO 30	N/A	M	<u> </u>	S- BULK SAMPLE		T. SOME FRAGMENTS OF STED, YIELDS SPT N VAL	STRONG ROCK USUALLY REMAIN.		LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MATERIA (NON-CO	HESTVE) DENSE	30 TO 50		ARTIFICIAL FILL ROADWAY EMBAN		SS- SPLIT SPOON SAMPLE			SCOLORED OR STAINED. ROCK FABRIC EL	EMENTS ARE DISCERNIBLE BUT	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN
	VERY DENSE	>50		INFERRED SOIL	BOUNDARIES &	ST- SHELBY TUBE	(V. SEV.) THE MA	ASS IS EFFECTIVELY RE	EDUCED TO SOIL STATUS, WITH ONLY FRA	GMENTS OF STRONG ROCK	SOILS USUALLY INDICATES POOR AFRATION AND LACK OF GOOD DRAINAGE.
GENERAL	VERY SOFT LY SOFT	2 2 TO 4	<0.25 0.25 TO 0.5	SIISIIS INFERRED ROCK	` MONITORING WE	ILL SAMPLE RS- ROCK SAMPLE			EXAMPLE OF ROCK WEATHERED TO A DE ROCK FABRIC REMAIN. <i>IF TESTED, YIELD</i>		PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
SILT-CLA		4 TO 8 8 TO 15	Ø.5 TO 1		A PIEZUMETER				FABRIC NOT DISCERNIBLE, OR DISCERNI		RESIDUAL SUIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
MATERIA (COHESI)	VE) VERY STIFF	15 TO 3Ø	1 TO 2 2 TO 4	TTTTT ALLUVIAL SOIL	BOUNDARY SLOPE INDICAT	RT- RECOMPACTED  TRIAXIAL SAMPLE		RED CONCENTRATIONS.	QUARTZ MAY BE PRESENT AS DIKES OR	STRINGERS, SAPROLITE IS	ROCK QUALITY DESIGNATION (R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF
	HARD	>30	>4	25/025 DIP/DIP DIRECTI	ION OF STALLATION	CBR - CBR SAMPLE	ACSO H	IN CAMPILLE.	ROCK HARDNESS		ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
	TEXTUR	E OR GRAIN SIZE		- Hook o'Mooron	SPT N-VALUE		VERY HARD CANNO	T BE SCRATCHED BY V	NIFE OR SHARP PICK, BREAKING OF HAN	COECIMENC DECITIONS	SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE
U.S. STD. SIEV		10 40 60 20		• - SOUNDING ROD	REF - SPT REFUSAL			RAL HARD BLOWS OF TH		SPECIMENS NEGOTINES	PARENT ROCK.
OPENING (MM)	4.76		0.053		ABBREVIATIONS				E OR PICK ONLY WITH DIFFICULTY, HARD	HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL
BOULDER	COBBLE GRAVEL	COARSE FII	ND SILI CLAY	AR - AUGER REF	USAL PMT - PRESSI	JREMETER TEST	1	TACH HAND SPECIMEN.	E OR PICK. GOUGES OR GROOVES TO 0.2	THOUSE DEED CAN BE	TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS
(BLDR.)	(COB.) (GR.)		SD.) (SL.) (CL.)	BT - BORING TE	RMINATED SD SAND, S4 SL SILT, SI		HARD EXCAV	ATED BY HARD BLOW O	OF A GEOLOGISTS PICK. HAND SPECIMENS		SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM SIZE IN.		2.0 0.25	0.05 0.005	CPT - CONE PEN	ETRATION TEST SLI SLIGHT	_Y	1	DDERATE BLOWS.	0.05 INCHES DEEP BY FIRM PRESSURE	OF WHITE OR DICK DOINT	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF
		- CORRELATION OF	TERMS	CSE COARSE DMT - DILATOME	TCR - TRICON				L CHIPS TO PEICES 1 INCH MAXIMUM SI		A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION
SOIL MO		D MOTOTURE	R FIELD MOISTURE DESCRIPTI	DPT - DYNAMIC	PENETRATION TEST 7 DRY III			OF A GEOLOGISTS PIC			WITH 60 BLOWS.
(ATTERB	ERG LIMITS) DES	SCRIPTION GOIDE FO		F FINE	w - MOISTURE		FROM	CHIPS TO SEVERAL INC	READILY BY KNIFE OR PICK. CAN BE EX CHES IN SIZE BY MODERATE BLOWS OF		STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
			LIQUID; VERY WET, USUALLY	FOSS FOSSILIF FRAC FRACTUR		HEAR TEST	1	S CAN BE BROKEN BY F	THOER TRESSORE.		OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (S.R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY:
LL	LIQUID LIMIT	(SAT.) FROM BE	LOW THE GROUND WATER TAB	FRAGS FRAGME		TEST			CAN BE EXCAVATED READILY WITH POIN BE BROKEN BY FINGER PRESSURE, CAN		TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC RANGE <			ID; REQUIRES DRYING TO	MED MEDIUM	IIDMENT LICED ON OUR IEST (	200 1507	FINGE				TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
(PI)	PLASTIC LIMIT	ATTAIN (	OPTIMUM MOISTURE	EUI	JIPMENT USED ON SUBJECT (			JRE SPACING	TERM BED	DING <u>THICKNESS</u>	
		COLID	AT OR MEAR ORTHWAY MOTOTIV	DRILL UNITS:	ADVANCING TOOLS:	HAMMER TYPE:	TERM VERY WIDE	. <u>SPACING</u> MORE THAN 10 FI	VERY THICKLY BEDDED	> 4 FEET	BENCH MARK: BM #2, THREE BARREL BOX CULVERT UNDER US-321; 35.57 RIGHT OF -BY- 11+45.94
OM +	OPTIMUM MOISTURE - M SHRINKAGE LIMIT	10IST - (M) SOLID;	AT OR NEAR OPTIMUM MOISTU	MOBILE B	CLAY BITS	AUTOMATIC MANUAL	WIDE	3 TO 10 FEET	THICKLY BEDDED THINLY BEDDED	1.5 - 4 FEET 0.16 - 1.5 FEET	S0.57 RIGHT OF -81- 11+45.94 ELEVATION: 2637.62
35	SHAINMOL CIPIT	REQUIRES	S ADDITIONAL WATER TO		6 CONTINUOUS FLIGHT AUGER	CORE SIZE:	MODERATELY CLOS CLOSE	SE 1 TO 3 FEET 0.16 TO 1 FEET	VERY THINLY BEDDED	0.03 - 0.16 FEET	NOTES:
	- [		OPTIMUM MOISTURE	BK-51	8 HOLLOW AUGERS		VERY CLOSE	LESS THAN 0.16	FEET THICKLY LAMINATED THINLY LAMINATED	0.008 - 0.03 FEET < 0.008 FEET	100123:
	P	PLASTICITY		CME-45	HARD FACED FINGER BITS	∑ -N			INDURATION		
	PLAST	TICITY INDEX (PI)	DRY STRENGTH		TUNGCARBIDE INSERTS	1	FOR SEDIMENTARY ROO	CKS.INDURATION IS THE	HARDENING OF THE MATERIAL BY CEME	NTING, HEAT, PRESSURE, ETC.	
NONPLASTIC	ITV	0-5	VERY LOW		. CASING W/ ADVANCER	н <u>о</u>	FRIABLE		RUBBING WITH FINGER FREES NUMEROUS		
LOW PLASTIC MED. PLASTIC		6-15 16-25	SLIGHT MEDIUM	PORTABLE HOIST		HAND TOOLS:			GENTLE BLOW BY HAMMER DISINTEGRATI		
HIGH PLASTIC		26 OR MORE	HIGH	- FORTABLE HOIST	TRICONE STEEL TEETH	POST HOLE DIGGER	MODERATEL	INDONHIED	GRAINS CAN BE SEPARATED FROM SAMP BREAKS EASILY WHEN HIT WITH HAMMER		
		COLOR		OTHER	TRICONE TUNG,-CARB.	HAND AUGER	INDURATED		GRAINS ARE DIFFICULT TO SEPARATE W	•	
DESCRIPT	IONS MAY INCLUDE COLOR OR O	COLOR COMBINATIONS (TAN, )	RED, YEL-BRN, BLUE-GRAY)		CORE BIT	SOUNDING ROD	MODINATED		DIFFICULT TO BREAK WITH HAMMER.		
MODIFIER	S SUCH AS LIGHT, DARK, STREAK	KED, ETC. ARE USED TO DES	CRIBE APPEARANCE.	OTHER	OTHER	VANE SHEAR TEST OTHER	· EXTREMELY		SHARP HAMMER BLOWS REQUIRED TO BR	EAK SAMPLE;	· ·
						U onex	1		SAMPLE BREAKS ACROSS GRAINS.		
											REVISED 09/15/00

PROJECT NO.

33356.1.1

SHEET NO. TOTAL SHEETS

ID B-3922



### STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY GOVERNOR

LYNDO TIPPETT SECRETARY

June 3, 2004

STATE PROJECT:

33356.1.1

TIP NO:

B-3922

COUNTY:

Watauga

**DESCRIPTION:** 

Bridge No. 316 on SR 1149 over Cove Creek

SUBJECT:

Geotechnical Report – Foundation Investigation

#### Site Description

Bridge number 316 is located in western Watauga County, approximately 7 miles west of Boone and 2.5 miles west of the junction of US 321 and US 421. The bridge is on SR 1149 at its intersection with US 321.

The existing bridge is to be replaced with a new structure on alignment -L- approximately 370 feet downstream. The new crossing is on a bend in the stream. A small tributary enters Cove Creek on the west bank about 100 feet upstream of -L-, flowing from the mouth of a 3-barrel concrete culvert beneath US 321.

Plans call for a new bridge constructed of pre-stressed girders in three spans of 50 feet, 65 feet and 40 feet, respectively, with a skew of 90 degrees.

Cove Creek is a major tributary of the Watauga River, with its headwaters on the flank of Rich Mountain, a 5000 foot ridge running north from the town of Boone to the Tennessee State Line. It is a turbulent, mountain stream 30 to 40 feet wide flowing on a bed of coarse sand, gravel and boulders, within banks 3 to 5 feet high. The flood plain at that point is 150 feet wide between the embankments of SR 1149 on one side and US 321 on the other side. An additional 100 feet of flood plain has been cut off by the US 321 roadway embankment. The floodplain is covered in grass and hay meadow, with a few scattered trees.

Cove Creek and the surrounding region experienced a moderate flood in the Fall of 2003, leaving evidence of its effects on the site of this project. The west bank of the creek on the proposed alignment was undercut and freshly eroded, as was the base of the

**MAILING ADDRESS:** NC DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT 1589 MAIL SERVICE CENTER RALEIGH NC 27699-1589

TELEPHONE: 919-250-4088 FAX: 919-250-4237

WEBSITE: WWW.DOH.DOT.STATE.NC.US

LOCATION: **CENTURY CENTER COMPLEX** BUILDING B 1020 BIRCH RIDGE DRIVE RALEIGH NC 27610 embankment on US 321 about 150 to 200 feet upstream of the proposed new crossing. The surface of the floodplain was eroded at new Bent Two and fresh deposits of gravel and small boulders were left. The edge of the channel migrated over the Right Side of Bent Two.

3 OF 17

The Geotechnical Engineering Unit conducted a Foundation Investigation in April, 2004. Borings were made with a CME 550 all-terrain power drilling machine equipped with a rotary casing advancer and NXWL diamond coring tools. Two borings were made at each bent, including one core boring at each interior bent. Standard Penetration Tests (SPT's) were made at 2.5 or 5 foot intervals in soil and weathered rock, and quality samples were taken from soil strata. Six rock core samples were submitted for compression tests and 2 samples for splitting tensile tests. All tests were conducted by a DOT laboratory.

#### Soil and Rock Materials

Borings at the site found embankment soils, alluvium, saprolite, weathered rock and hard rock. Alluvial soils on the floodplain directly overlie weathered rock and hard rock. Saprolite was found underlying weathered rock and hard rock in one boring.

Alluvial soils are 7 to 9 feet thick and are typically composed of a few feet of dark brown. very soft silt (A-4) or clay (A-7) overlying loose sand and gravel with cobbles and boulders (A-1-b). Alluvium on the west bank around Bent Two is all loose sand and gravel (A-1-b) or silty sand (A-2-4) with suspended gravel.

Embankment soil is found only at End Bent One, where an old roadway embankment gives access to the floodplain from SR 1149. The soil is very soft, yellow-brown sandy silt (A-4) with suspended rock fragments and a few boulders.

The rock at this site comprises a variety of hard, crystalline lithologies, the most abundant of which are layered biotite gneiss and massive biotite gneiss. The layered biotite gneiss is composed of dark gray, biotite-rich layers alternating with light gray feldspar-rich layers .05 to .80 feet thick. Those biotite gneisses display moderately well-developed foliation that dips 25 degrees, parallel with the layering where present. Four additional lithologies are part of the stratified sequence found at Bent Two. They are light gray feldspathic gneiss; very coarse, white crystalline pegmatite; a feldspathic transition between the pegmatite and biotite gneiss with a well-developed shear foliation; and metagabbro composed chiefly of hornblende partially altered to chlorite and moderately well foliated. The foliation in those lithilogies dips 10 to 20 degrees. Rock quality among all those lithologies varies from very poor to very good.

#### **Bent Descriptions**

End Bent One (EB1): This bent lies across an embankment beside the shoulder of SR 1149. The embankment is part of an old soil road from SR 1149 to the floodplain surface. The Left Side of the bent is on the surface of the floodplain and the Right Side is on the embankment.

A boring on the Left Side (EB1-A) penetrated alluvial soil consisting of 5.3 feet of very soft, sandy silt (A-4) overlying 2.9 feet of gravel with boulders or cobbles (A-1-b). Weathered rock was encountered at the base of the gravel at a depth of 8.2 feet. The bit continued through weathered rock to hard rock at 15.6 feet and then to termination with SPT spoon refusal at 19.0 feet. Static groundwater was found in this boring at elevation 2631.5 feet.

A boring on the Right Side (EB1-B) penetrated 8.0 feet of embankment soil consisting of very soft, sandy silt (A-4) with suspended rock fragments and a few boulders near the base. The embankment overlay alluvium composed of 4.0 feet of dark gray, very soft clay (A-7-5) grading downward to silt and sand near the base, then 2 feet of basal, loose, coarse sand and gravel (A-1-b). Weathered rock was encountered at the base of the gravel at 14.0 feet. The bit passed into hard rock at 15.2 feet. At 17.0 feet the bit broke out of hard rock into hard, silty saprolite (A-4) and continued in saprolite to a depth of 24.7 feet, where it entered weathered rock again. The boring was terminated in weathered rock at 25.0 feet. The hole caved before the static water table could be identified.

Bent One (B1): This bent lies on the floodplain and is centered about 30 feet east of the stream bank. A large willow tree is located on the Right Side of the bent. A boring on the Left Side (B1-A) penetrated 5.0 feet of alluvial, very soft, sandy silt (A-4) and 2 feet of basal gravel (A-1-b) to encounter weathered rock at a depth of 7.0 feet. The boring passed into hard rock at 7.8 feet and was terminated with SPT spoon refusal in hard rock at a depth of 9.9 feet. Static groundwater was found at elevation 2629.9 feet.

A boring on the Right Side (B1-B) penetrated 3.2 feet of alluvial, very soft, sandy silt (A-4) and 4.0 feet of alluvial gravel (A-1-b) to encounter weathered rock at a depth of 7.2 feet. The boring was carried through weathered rock with hard seams to hard rock at 9.2 feet, and continued. The core barrel was inserted at 10.9 feet and hard rock was cored from that point to termination at 25.7 feet. Coring recovered two lithologic strata of fresh, hard rock: an upper unit of layered biotite gneiss 6.7 feet thick (REC=94% RQD=87%) and a lower unit of massive biotite gneiss (REC=98% RQD=96%). Static groundwater was found in this boring at elevation 2631.2 feet.

Bent Two (B2): This bent is located on the west bank of the stream, with its Left Side on the floodplain and its Right Side in the stream channel. A boring on the Left Side (B2-A) found 9.0 feet of alluvial coarse sand and gravel (A-1-b), with boulders near the base, overlying weathered rock. The boring continued in weathered rock to 11.0 feet, at which point the core barrel was inserted, and hard rock was cored from that point to a final depth of 38.7 feet. A variety of hard, crystalline lithologies was penetrated, as given in the following table with depths and rock quality:

- 11.0-13.7 moderately hard meta-gabbro, good quality
- 13.7 16.6 hard, slightly weathered, sheared pegmatite, very poor
- 16.6 21.2 hard, fresh pegmatite, very good
- 21.2 31.2 hard, slightly weathered, feldspathized transition from pegmatite to layered biotite gneiss, sheared, very poor (loss to core barrel malfunction)

- 31.2 33.2 hard, fresh felsic gneiss, good
- 33.2 35.9 moderately hard, fresh meta-gabbro, very good
- 35.9 38.7 hard, fresh layered biotite gneiss, very good

The boring caved at a shallow depth before the static groundwater table could be identified.

A boring on the Right Side (B2-B) found 11.2 feet of alluvium composed of 2.5 feet of surficial sand, gravel and cobbles (A-1-b) overlying 7.7 feet of loose to medium dense, silty sand with suspended pebbles (A-2-4) and 1.5 feet of basal gravel and boulders (A-1-b). The boring penetrated weathered rock with hard rock seams from the base of alluvium at 11.2 feet to continuous hard rock beginning at 19.0 feet, and the boring continued in hard rock to a termination depth of 23.1 feet. The static groundwater table was found at an elevation of 2631.1 feet.

### End Bent Two (EB2)

This bent lies on the floodplain at the base of the US 321 embankment. Borings on the Left Side (EB2-A) and Right Side (EB2-B) recorded similar results. Alluvial soils were 7 to 8 feet thick composed of 1 to 2.5 feet of very loose, sandy silt (A-4) overlying 5 to 6 feet of gravel A-1-b). Beneath the alluvium, the borings encountered a thin layer of weathered rock 1 to 2.5 feet thick, with hard rock seams. Reliable hard rock was found at approximately 9.5 feet in both borings, and the borings were terminated in hard rock at 11.7 and 11.3 feet, respectively. The static ground water table was found at 1.2 feet in EB2-A and at 2.0 feet in EB2-B.

Respectfully submitted,

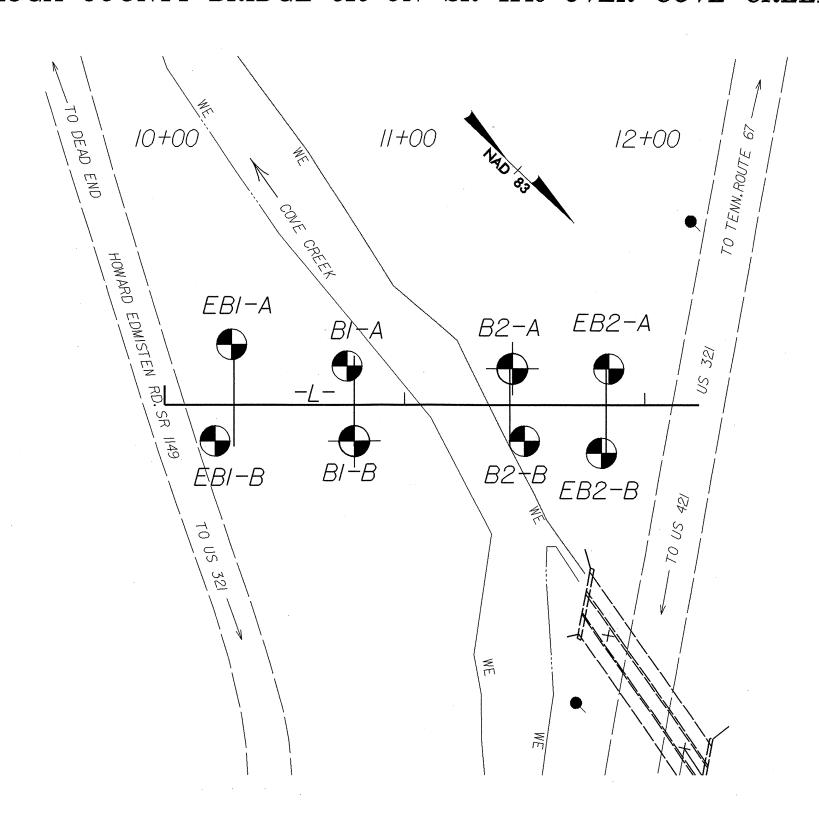
OF 17

Louis L. Acker, LG Project Geologist

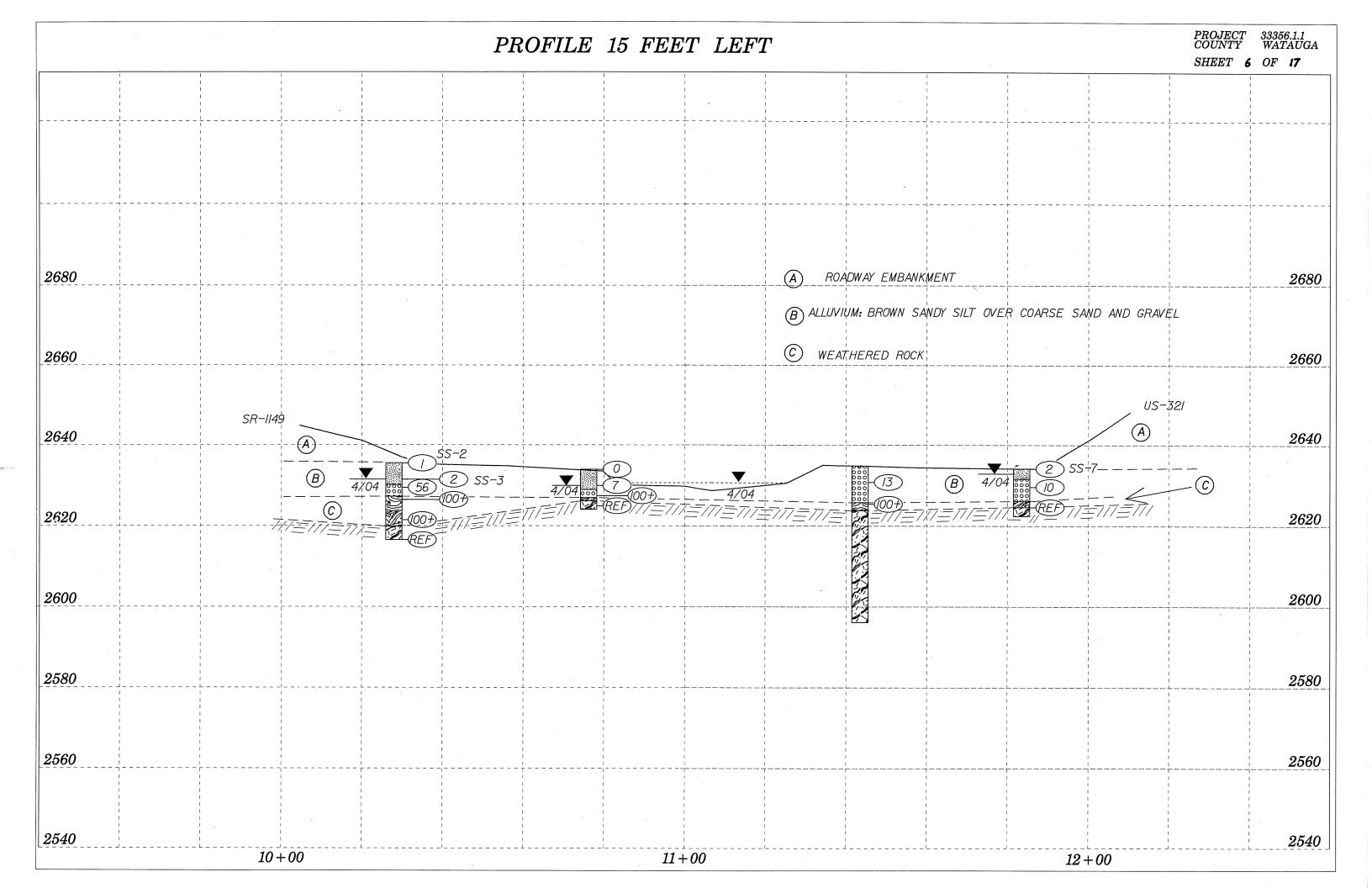
### SHEE

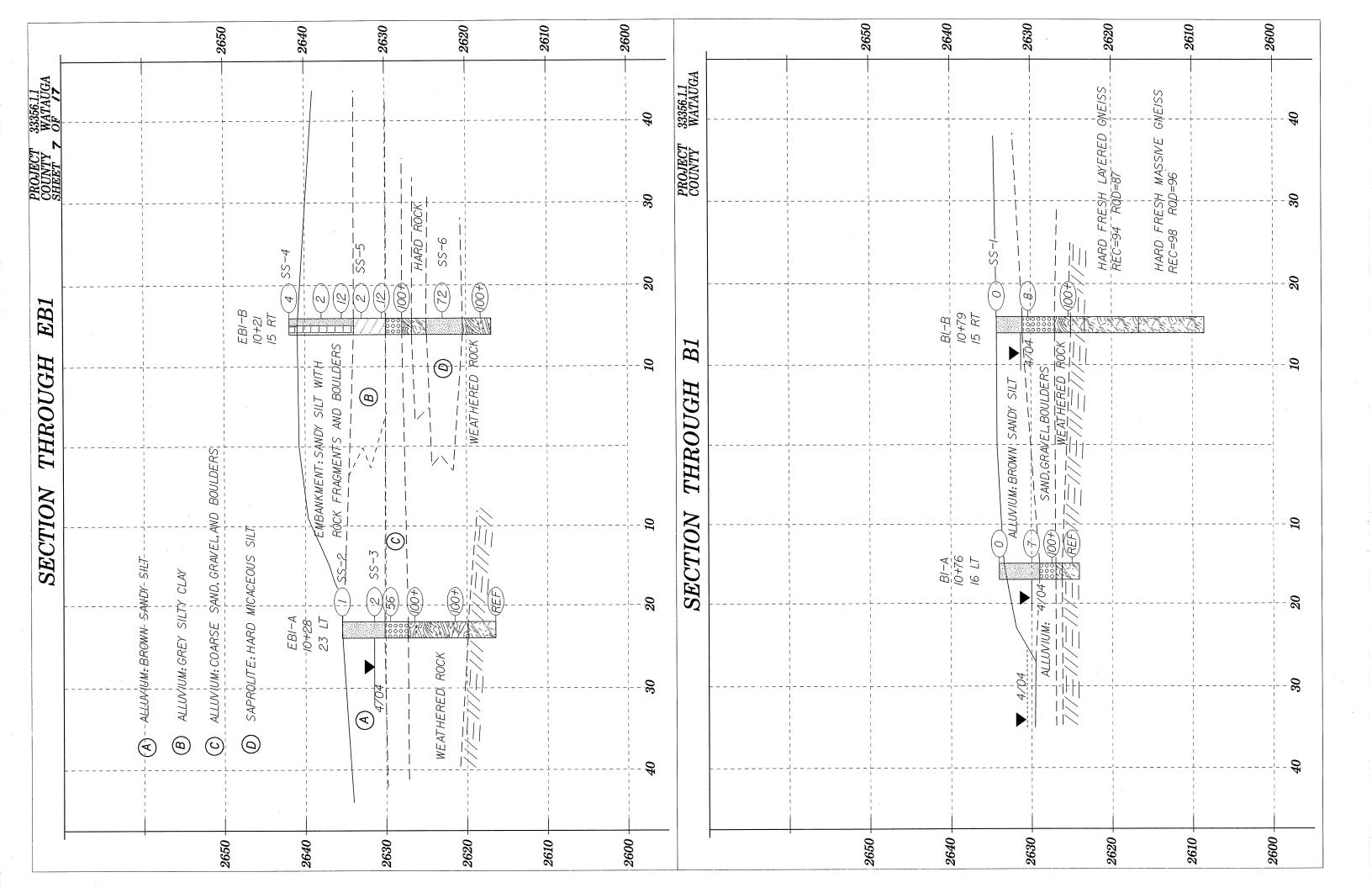


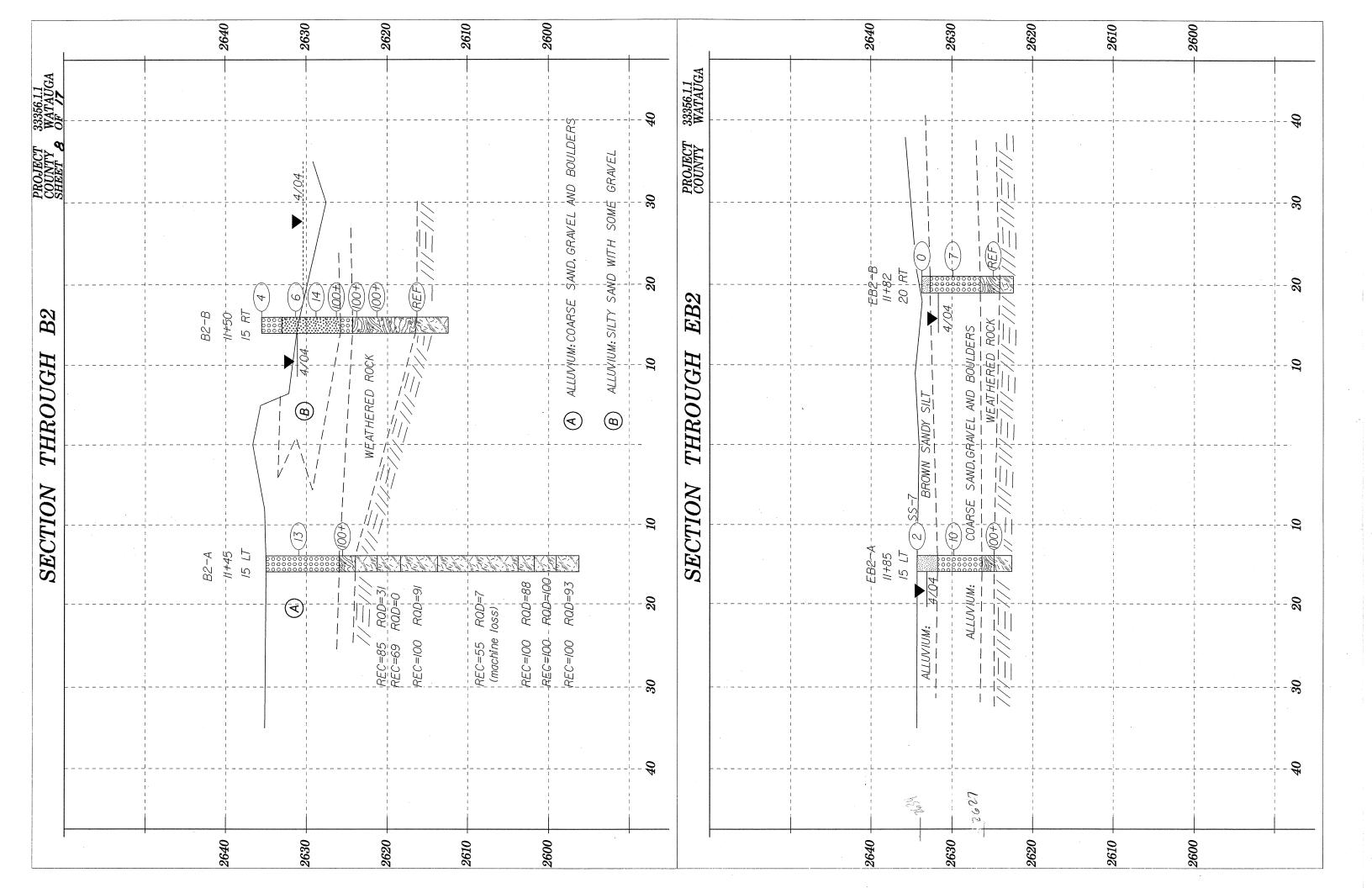
# BORING LOCATIONS WATAUGA COUNTY BRIDGE 316 ON SR 1149 OVER COVE CREEK



0 40 80 SCALE IN FEET







### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

					G	<b>EOTE</b>	<u>CHNI</u>	<u>CAL I</u>	JNIT E	ORING	3 LO	G		
PROJEC	T NO 33	356.1	.1	I	DB-	3922	COU	NTY W	ATAUGA	1	GEO	LO	GIST L. L. ACKE	R
SITE DES	SCRIPTI	ON E	RIDO	GE N	O. 31	6 ON SR	1149 O	VER CO	OVE CRE	EK				GND WATER
BORING	NO EB1	<u>-A</u>		1	ORT	HING 0.	.00			EASTING	G 0.00	)		OHR N/A
ALIGNM						NG LOC				OFFSET		ft L	· · · · · · · · · · · · · · · · · · ·	24 HR 4.00ft
COLLAR					OTA	L DEPTI				ATE 4/20		<u> </u>	COMPLETION	DATE 04/20/04
DRILL M				)		····	<del></del>			ARY W/O	MUD		HAMMER TYP	
SURFAC	E WATE		OW.	CT	PEN	Б		TO ROPER FO	OCK 15.6	SAMPLE	1 — 7	111	Log EB1-A, Page 1 o	f1 ID ROCK
ELEV	DEPTH		6in						75 10	•	MOI	١۵۱		RIPTION
		Oiii	Oiii	0111	(11)	<del> </del>	<u> </u>	<del> </del>	1	110	IVIOI	G	DESCR	AIF HON
] -	_													
-														
_	_													
	_												•	
	_													
-	-							:		· ·				
2635.50	0.00	0	0	1	1.0		Ground	Surfac	e	<u> </u>				
_	- -			44.		<b>X:1:::</b>				SS-2	M		ALLUVIUM: BRO	WN SANDY SILT
	4.00	0	0	2	1.0	-2				000	W			
2630.00_	6.50	18	12	44	1.0			56		SS-3	VV	0000 0000 0000 0000	ALLUVIUM: CO	DARSE SAND.
_	9.00	100			0.4			X	100		,	0000 0000	GRAVEL ANI	D BOULDERS
	- 								<u> </u>	9			WEATHERED F	ROCK: BIOTITE
	-											S	GIN	-100
h00000	14.00	58	42		0.9									
2620.00									11	1		N	HARD ROCK: F	IOTITE GNEISS
2616.50	- - 19.00	100			0.0				100-	J				
	_					ERMIN	TED B	DRING:	NHARI	1				
	- -				K	ΨGK A F	ELE <del>VA</del> 	HON 20	16.5 FE	<b>1</b> '				
	_													
-	-													
_	_									· ·				
_	_													
_	<u> </u>													
-	_													
	_													
·[ -	_													
-	_												•	
											·			
-	-													
													,	
-														
] -	-													1
	_	-	1	l .									$\frac{d}{dt} = \frac{dt}{dt} + \frac{dt}{dt} = \frac{dt}{dt} + \frac{dt}{dt} = \frac{dt}{dt} + \frac{dt}{dt} = \frac{dt}{dt} + \frac{dt}{dt} = \frac{dt}$	· · · · · · · · · · · · · · · · · · ·
	_							:					•	
	<del>-</del>				•					1	`			
	 -													
	<del>-</del> -													
	-							:						
	-								-	Ī				

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

	***************************************			***************************************	G	EOTE	CHNI	CAL	JNIT E	BORING	<u> LO</u>	G		
PROJEC					D B-				ATAUG		GEO	LO	GIST L. L. ACKE	R
SITE DE			RID					VER CO	OVE CRE	EK				GND WATER
BORING						HING 0.				EASTING	G 0.00	)		0 HR N/A
	IENT -L-					NG LOCA				OFFSET		ft R		24 HR N/A
	RELEV 2		***************************************		OTA	L DEPTI				ATE 4/20			COMPLETION	DATE <b>04/20/04</b>
DRILL N				~~~~						ARY W/O	MUD		HAMMER TYP	
SURFAC	T		OW.		PEN	R		TO ROPER FO	OCK N/A	SAMPLE		111	Log EB1-B, Page 1 o	f 1 ID ROCK
ELEV	DEPTH		l 6in						75 10	_1	MOI	١۵۱		RIPTION
_	L	-	-	-	(1.0)			<u> </u>	1	1.0	IVIOI	19	DESCR	.IF I ION
<u> </u>	Ł			İ										
1 :	Ł													
-	<del>L</del>										·			
-	Ł													
_	_													
-	E									,				
_	F													
-	F												•	
2641.90 I	0.00	1	3	1	1.0		Ground	Surfac	6	55-4	M	126		
2640.00_	<b>F</b>					<b>X</b> 4				33-4	141	E	MBANKMENT: Y	ELLOW-BROWN TH FEW ROCK
-	4.00	1	1	1	1.0	]_2						E	FRAGMENTS	TO BOULDERS
-	6.50	9	9	3	1.0	12						E		,
-	<b>†</b>					-X:								
-	9.00	1	1	1	1.0	W				SS-5	W		ALLUVIUM: DAF	RK GRAY SILTY
2630.00 <u> </u>	11.50	0	6	6	1.0	12						$\square$		AY
-	14.00	71	29		0.9				100			0000 0000	ALLUVIUM: CO	DARSE SAND,
	-								13	9				BOULDERS RED ROCK
-	19.00	47	40	32	1.0				75				·	ROCK
-	19.00	47	40	32	1.0			3		SS-6	М		SAPROLITE: G	
2620.00 <u> </u>				alterno								33	MICACE	OUS SILT
- 2616.90 -	23.70	27	30	70	0.7								WEATHER	RED ROCK
2010.90 -						TERN	HNATE	BORI	NG IN -	1		24		
-						I WE	THERE	DROC 2616.9	KAT					
-	<b>F</b>					ELEV	AHUN	20.10.9	TEE H					
	<b>_</b>													
_	F													:
-	F									,				
_	F													
-	F													
_	<b>F</b>													
-	F												• .	
-	‡ .													ı
-	‡													
	‡				7									
_	<b>L</b>			<b>.</b> .										
-	‡										l .			
-	‡	ŀ											•	
-	<b>‡</b>													
-	<b>‡</b>													**
	<u> </u>													
-	<b>t</b>													V
	I	L	<u> </u>	<u></u>		<u> </u>		<u> </u>	1		<u> </u>		V	

### 10 OF 17

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

					G	EOTE	CHNIC	CALL	JNIT E	ORING	S LO	G		
PROJEC					D B-3				ATAUG/		GEO	LO	GIST L. L. ACKE	R
SITE DE	SCRIPTI	ON E	RIDO	GE N	0. 31	6 ON SR	1149 O	VER CC	VE CRE	EK		-		GND WATER
BORING	NO B1-	4		1	NORT	HING 0.	.00			EASTING	G 0.00	)		0 HR N/A
ALIGNM	ENT -L-			I	BORI	NG LOCA	ATION '	10+76.0	00	OFFSET	16.00	ft L	T	24 HR 4.00ft
COLLAR	RELEV 2	634.0	00ft	7	ГОТА	L DEPTI	H 9.90ft	5	TART D	ATE 4/20	/04		COMPLETION	
DRILL M	IACHINE	CM	E 550	)			DRILL	METH	OD ROT	ARY W/O	MUD		HAMMER TYP	
SURFAC	E WATE					·	DEPTH	I TO RO	CK 7.80	)ft			Log B1-A, Page 1 of	
ELEV	DEPTH		.OW		PEN		LOWS F	PER FO	TC	SAMPLE	Y/	151	SOIL AN	D ROCK
		6in	6in	6in	(ft)	0 2	.5 5	0 7	5 10	NO P	<b>▼</b> /MOI	g	DESCR	RIPTION
_	_				·							П		
_	-			l										
_	-													
_	-													
_	-													
	-													
-													;	
2634.00 <sup>-</sup>	0.00	0	0	0	1.0		Sround	Surface						
2034.00 _		,	<u> </u>			¥0	Diodila	Juliace					ALLINGUA. D	A DIV DDOUAL
	-			_	l I	<u> </u>							ALLUVIUM: DA	Y SILT
2630.00	_4.00	1	2	5	1.0	7-7-					Y			
_	6.50	22	78		0.5				100_			0000 0000	ALLUVIUM: CO	DARSE SAND.
_	9.00	100		-	0.1				100				GRAVEL AND	BOULDERS /
2624.10							TED D					<b>%</b>		RED ROCK
_	-				R	ERMINA CK AT	FI FVA"	PRING+	N-HAKU DA 1 FF	h .			HARD ROCK: B	IOTITE GNEISS
_	_									<b>'</b> '				
-	-			,										ĺ
-														
·												1		
														i
-	_													
_	_													
			r											
=	- '												•	
	_													
-														
	_													1.
-		-											•	
_	_													
=														•
	_													
7	-													
	-													
_	_													
=													•	
	-			,										
. =	_													1
7	-													l
=	-			-										
	_												•	
-	-													

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

										ORING		_		
												LO	GIST L. L. ACKE	R
			RIDO					VER CO	VE CRE					GND WATER
BORING	NO B1-I	3		1	ORT	HING 0.	.00			EASTING	<del>3</del> 0.00	)		0 HR N/A
ALIGNM	ENT -L-			I	BORI	NG LOCA	ATION '			OFFSET		ft R	T	24 HR 3.00ft
COLLAR	ELEV 2	634.2	20ft	1	OTA	L DEPTI	1 25.70f	t s	TART D	ATE 4/19	/04		COMPLETION	DATE 04/19/04
DRILL M	IACHINE	CM	E 550	)			DRILL	METHO	OD SPT	CORE BO	RING	<u>;                                    </u>	HAMMER TYP	E AUTOMATIC
SURFAC	E WATE	R DE	PTH						CK 9.20	ft			Log B1-B, Page 1 of	1
ELEV	DEPTH		.OW		PEN		LOWS P			SAMPLE	MOI		•	D ROCK
		6in	6in	6in	(ft)	0 2 1	.5 5 L	i0 7	5 100 I I	NO	MOI	Ğ	DESCR	RIPTION
-	-													
-	-													
-	_								[]					
	_												2	<i>.</i>
_														
_														
0004.00	0.00	0	0	0	1.0		Ground	Surface						
2634.20	0.00		Ť			<u> </u>				55-1	M		ALLUVIUM: BRO	WN SANDY SILT
	- - 3.90	3	3	5	1.0	<u></u>					Y			
2630.00_	_3.90	ა	3	3	1.0	X 8						0000	ALLUVIUM: SILTY AND BO	SAND, GRAVEL
-	_											0000	AND BO	ULDERS
_	8.90	100			0.5				100-				WEATHERED RO	
					.				<b>-</b>				ROCK	/
_	_									CORE 1		図	HARD ROCK	
2620.00_										CORE 2			HARD ROCK: I LAYERED BIC	HARD, FRESH,
													REC=94	
-	_									2005.0				
_	_									CORE 3			HARD ROCK: I MASSIVE BIO	TITE GNEISS
_												認	REC=98	
2610.00_	-									CORE 4				
2608.50 -	_											24	·	
_	<u> </u>					ERMINA OCK AT-	TED B	DRING	NHARD					
							ELEVA			1.				
_	_													•
	_									,				
_												ll		
_		•												
_														•
-	_													
] -														
] -	F	,				[								
	-								,					
													•	·
] =	Ė l													
	_											•		•
	-	-												
	-  -													
	_													
]	_					L								
]	F					[								
-	-													
_	-													

PROJECT NO: 33356.1.1 (B-3922) WATAUGA COUNTY

### CORE BORING REPORT B1-B

CORE 1: 10.9 – 13.2	REC=91% RQD=70%
CORE 2: 13.2 – 18.2	REC=94% RQD=87%
CORE 3: 18.2 – 23.2	REC=100% RQD=100%
CORE 4: 23.2 – 25.7	REC=96% ROD=92%

LAYER 1: 10.9 – 17.6 Hard, fresh, layered biotite gneiss with mylonitic texture. Layers are dark, biotite-rich and light, feldspar-rich 0.05 to 0.8 feet thick. 13 pieces, longest piece 1.3 feet. Well foliated. Layering and foliation are parallel, dipping 25 degrees. 8 joints on foliation, smooth to moderately rough, coated with mica. 2 joints at 10-20 degrees opposite to foliation, rough, clean. 2 joints at 60 degrees, rough, clean. REC=94% RQD=87%

LAYER 2: 17.6 – 25.7 Hard, fresh, massive biotite gneiss with porphyroclastic mylonitic texture. 9 pieces, longest piece 2.0 feet. Foliated at 20 degrees. 6 joints on foliation, moderately rough, coated with a little mica. 1 joint at 40 degrees, rough, clean. REC=98% RQD=96%

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

										ORING				
PROJEC					D B-3				ATAUGA	<del></del>	GEO	LO	GIST L. L. ACKE	
SITE DES			KIDO				<del></del>	VER CO	IVE CRE		~ ^ ^			GND WATER
BORING		4				HING 0.		44.45.0		EASTING				0 HR N/A
ALIGNM			2051			NG LOCA		·····		OFFSET		II L	·	24 HR N/A
COLLAR					IUIA	L DEPTI	<del></del>			ATE 4/20			COMPLETION	
DRILL M				<u>,                                    </u>			\$			AUGERS			HAMMER TYP	
SURFAC			OW	CT	IPEN	R		PER FO	OCK 11.0	SAMPLE			Log B2-A, Page 1 of	D ROCK
ELEV	DEPTH		6in		1 1				75 100		MOI	Ω	DESCR	
		OIII	OIII	0111	1117	<del></del>		<del>-</del>	<del>i i</del>	1 110	INIOI	9	DESCR	IF HON
.] -	_										,		• ,	
	-						-,							
													•	
	_													
_							 Ground	Confo	<u></u>					
2635.00			<u> </u>	<b> </b>	$\vdash$		Ground	Suriace				0000	ALLUVIUM: SAN	D AND CDAVEL
		٠.										0000	TO BOL	ILDERS
2630.00_	_ 4.00	18	6	7	1.0	C						0000		
_	_					*						0000 0000		
									100-			0000		
	- 9.40 -	100			0.2				<del> </del>	4		<u> </u>	WEATHER	ED ROCK
										CORE 1			HARD ROCK: M	
	_									CORE 2			WEATHERED N	IETA-GABBRO
2620.00_										CORE 2			REC=85	RQD=31/
	-												HARD ROCK	HARD, SLI.
-	_									CORE 3			WEATHERED REC=69	
	<del>-</del>												HARD ROCK: I	
	<del>-</del>												PEGMATITE RE	C=100 RQD=91
2610.00_										CORE 4			HARD ROCK	
	_												WEATHERED TRANSITION FROM	ON DECMATITE
	- -									CORE 5			TO BIOTITE GN	
	-				.					OOKE 3		<b>10</b>	RQD=7 (MAC	HINE LOSS)
	_												HARD ROCK: I	HARD, FRESH
2600.00_				·						CORE 6		33		EC=100 RQD=88
-	_												HARD ROCK: I META-GABBI	HARD, FRESH
2596.30												<b>3</b>	RQD	
-	_					ERMIN	TED B	DRING I	NHĀRĪ			١	HARD ROCK: H	HARD, FRESH,
1 -	_				K	JUK AT	ELEVA	10N-25	96.3 FE	1.			LAYERED BIO REC=100	TITE GNEISS /
_	_												KEG-100	NGD-93
_	-													
_	-													
_														
	_													
	<u> </u>													
-	_													
	_								<u> </u>		,			
-	_							<u></u>						
1 7	<u> </u>													
	_													
			<u> </u>					L						

12 OF 17

PROJECT NO: 33356.1.1 (B-3922) WATAUGA COUNTY

### CORE BORING REPORT B2-A

CORE 1: 11.0 – 13.7	REC=85%	RQD=31%
CORE 2: 13.7 – 18.7	REC=82%	RQD=42%
CORE 3: 18.7 – 23.7	REC=82%	RQD=56%
CORE 4: 23.7 – 28.7	REC=33%	RQD=0%
CORE 5: 28.7 – 33.7	REC=95%	RQD=58%
CORE 6: 33.7 – 38.7	REC=100%	ROD=97%

LAYER 1: 11.0-13.7 Moderately hard, slightly weathered, hornblende-chlorite metagabbro, massive, highly fractured, well-foliated at 20 degrees. Severely weathered at base. >20 pieces, longest piece 0.9 feet. Joints indeterminate, most at 20 degrees or 70 degrees, clean or coated with Fe-oxide. REC=85% RQD=31%

LAYER 2: 13.7 – 16.6 Hard, slightly weathered, fractured white pegmatite. Poorly developed micaceous shear cleavage at 20 degrees. 30 pieces, longest piece 0.3 feet. Joints indeterminate and numerous due to weathering on shear cleavage, mica coated. REC=69% RQD=0%

LAYER 3: 16.6 – 21.2 Hard, fresh, white pegmatite. 5 pieces, longest piece 1.5 feet. Thin, micaceous shear planes dipping 20 degrees, most not open. 5 joints at 20 degrees on shear planes, smooth to moderately rough, coated with mica. REC=100% RQD=91%

LAYER 4: 21.2 – 31.2 Hard, slightly weathered transition from sheared pegmatite to layered biotite gneiss. 33 pieces, longest piece 0.45 feet. Poorly foliated and more or less layered, dipping about 20-25 degrees. 14 joints on foliation, moderately rough, clean or with a little Fe-oxide. 8 joints at 0-10 degrees, moderately rough, clean. One joint at 80-90 degrees, rough, coated with chlorite. Severely weathered seam at basal 0.1 foot. Most core loss due to core barrel malfunction. REC=55% RQD=6.5%

LAYER 5: 31.2 – 33.2 Hard, fresh felsic gneiss. 3 pieces, longest piece 1.75 feet. Very poorly foliated at 10 degrees. 3 joints on foliation close together, smooth, clean. REC=100% RQD=88%

LAYER 6: 33.2 – 35.9 Moderately hard, fresh, hornblende-chlorite meta-gabbro. 2 pieces, longer piece 2.2 feet. Well foliated at 10 degrees. REC=100% RQD=100%

LAYER 7: 35.9 – 38.7 Hard, fresh, layered biotite gneiss. 8 pieces, longest piece 0.9 feet. Foliated parallel with layering at 10 degrees. Layers are 0.05 to 0.5 feet thick. 7 joints on foliation, smooth, clean. REC=100% RQD=93%

#### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **GEOTECHNICAL UNIT BORING LOG** PROJECT NO 33356.1.1 ID B-3922 COUNTY WATAUGA **GEOLOGIST L. L. ACKER** SITE DESCRIPTION BRIDGE NO. 316 ON SR 1149 OVER COVE CREEK **GND WATER** BORING NO B2-B NORTHING 0.00 EASTING 0.00 0 HR N/A ALIGNMENT -L-**BORING LOCATION 11+50.000** OFFSET 15.00ft RT 24 HR 4.40ft COLLAR ELEV 2635.50ft TOTAL DEPTH 23.10ft **START DATE 4/21/04 COMPLETION DATE 04/21/04 DRILL MACHINE CME 550** DRILL METHOD ROTARY W/O MUD HAMMER TYPE AUTOMATIC SURFACE WATER DEPTH N/A DEPTH TO ROCK 19.00ft Log B2-B, Page 1 of 1 **BLOW CT** PEN **BLOWS PER FOOT** SOIL AND ROCK DEPTH **ELEV** Moi & 6in | 6in | 6in | (ft) NO **DESCRIPTION** 2635.50 ALLUVIUM: SAND AND GRAVEL TO BOULDERS 4.20 3 3 1.0 Y ALLUVIUM: SILTY SAND WITH A LITTLE GRAVEL 2630.00\_ 6.70 11 8 6 1.0 16 100 9.20 0.2 **ALLUVIAL: BOULDERS** 11.70 70 31 69 0.9 **WEATHERED ROCK AND HARD** 100 14.20 100 0.2 **ROCK SEAMS** 2620.00\_ 19.20 | 100 0.1 HARD ROCK 2612.40 TERMINATED BORING IN HARD ROCK AT ELEVATION 2612.4 FEET

13 OF 17

#### 14 OF 17

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

					G	EOTE	CHNIC	CALL	JNIT B	ORING	LO	G		
PROJEC	T NO 33	356.1	.1	I	D B-	3922	COU	NTY W	ATAUGA		GEO	LO	GIST L. L. ACKE	R
SITE DES	SCRIPTION	ON E	RIDO	SE N	O. 31	6 ON SR	1149 O\	VER CO	VE CRE	EK				GND WATER
<b>BORING</b>	NO EB2	-A		1	ORT	HING 0.	00			EASTING				0 HR N/A
ALIGNM				1	BORI	NG LOCA	ATION 1	1+85.0	00	OFFSET	15.00	ft L	<u>.T                                      </u>	24 HR 1.20ft
COLLAR	ELEV 2	634.3	30ft		ГОТА	L DEPTI	I 11.70f	t s	TART D	ATE 4/21	/04		COMPLETION	DATE 04/21/04
DRILL M	IACHINE	CM	E 550	)			DRILL	METH	DD ROTA	ARY W/O	MUD		HAMMER TYP	E AUTOMATIC
SURFAC	E WATE	R DE	PTH						CK 9.50	ft			Log EB2-A, Page 1 o	f 1
ELEV	DEPTH		.OW		PEN		LOWS P			SAMPLE	MOI	P		ID ROCK
		6in	6in	6in	(ft)	1 -	5 5 	0 7	5 100	NO	MOI	G	DESCH	RIPTION
_														
_														•
_		-											·	
_	_													
_	F												•	
-	_													
	_													
2634.30	0.00	0	0	2	1.0		Ground	Surface						
2034.30						2		<del></del>		SS-7	Y		ALLUVIUM: D	ARK BROWN.
		1				18				12.7		0000	SAND	Y SILT
2630.00_	4.50	4	3	7	1.0	- \10						0000 0000 0000 0000	ALLUVIUM: SAN COBBLES V	D, GRAVEL AND
ν; -	-					-						0000	COBBLES V BOUL	VITH BASAL
22	0.50	400							100			0000 0000 0000		
25	9.50	100			0.1				:::::*	1				RED ROCK
2622.60 -						ERMIN	TED P	DIMO					HARD	ROCK
_					R	OCK AT	ELEVA	10N-26	22.6 FEE	r.				
-	-				'`	1				•				
_	-													
_	_													
_	<b>-</b>										Ì			
_														
_	_									1				
_	_													
_														
_														•
-												l		
] -	-													
] -														•
-														
	-													
													,	
_	E				ľ									
_	E													
] -	F													
-	F								[]					g en
-	F								[]				'	
-	t i		1											
-	<u> </u>													
-	-		·	l										
_	F		1	1										
	<u> </u>		l	<b>.</b>					[]					
-	F		1	'					[]					

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

					<u>G</u>	EOTE	CHNIC	CAL L	INIT B	ORING	LO	G		
	T NO 33				D B-				ATAUG <i>A</i>		GEO	LO	GIST L. L. ACKE	R
	SCRIPTI		RID	GE N	0. 31	6 ON SR	1149 O	VER CO	VE CRE	EK			,	GND WATER
	NO EB2					HING 0.				EASTING	3 0.00			0 HR N/A
	IENT -L-					NG LOCA				OFFSET		ft R	T	24 HR 2.00ft
	RELEV 2				OTA	L DEPTI	1			ATE 4/21			COMPLETION	
	<b>IACHINI</b>			)						ARY W/O	MUD		HAMMER TYP	
	E WATE	R DE	.OW	СТ	PEN		DEPTH LOWS P	TO RO	CK 9.60	oft SAMPLE		_	Log EB2-B, Page 1 o	f1
ELEV	DEPTH	8	6in						5 10	1	MOI	ձ		D ROCK
	<u> </u>	0		-	(10)	<del> </del>	-			110	MICI	G	DESCR	RIPTION
_	E													
_														
	F								10000		,			
	F										-			
	<u> </u>										-			
-	‡ I													
-	F '													
2633.70 -	0.00	0	0	0	1.0		<u>Ground</u>	Surface						
-	‡					<b>*</b> •					Y	0000 0000 0000	ALLUVIUM: D.	
2630.00 <u> </u>	3.80	1	2	5	1.0	7						0000		Y SILT
- A	‡					X:::						0000	ALLUVIUM: SANI COR	O , GRAVEL AND BLES
- c3-	<u> </u>											0000 0000 2 4		
2,	8.80	100			0.3				K	4			WEATHER	RED ROCK
2622.40 -	<u> </u>											32	HARD	ROCK
-	<u> </u>				R	IJEKWIN/ OGK-AT	ELENY:	DRING I	NHARI 2.4 FEE	+				
-	-				']			1011202		[				
-	L													
-									<u></u>					
-	<u> </u>													
_	_													•
	E													, ,
_			÷											,
_														
_	_												•	
	_													
-	E													
-														
_											,			
-	E										-			
-	<u> </u>													•
_	E									,				,
-	F													
-	F													
	F		,											
-	F					F								
-	_													
-	‡													
-	<b>L</b>			-										
-	<b>†</b>			l										
-	‡													*
-	<b> -</b>													
_	<u> </u>			<u></u>		<u> </u>								

#### GEOTECHNICAL ENGINEERING UNIT FIELD SCOUR REPORT

PROJECT: <u>33356.1.1</u>	ID: <u>B-3922</u>	COUNTY:	WATAUGA		
DESCRIPTION(1):	BRIDGE NO. 316 ON SR 114	9 OVER COV	E CREEK		
INFORMATION ON EX	(ISTING BRIDGES Informa	ation obtaine	d from: X	field inspection	
				microfilm(Reel:other	_ Pos:)
COUNTY BRIDGE NO.	316 BRIDGE LENGTH 60 FT	NO. BENTS		•	
FOUNDATION TYPE:		34 m			
EVIDENCE OF SCO					
ABUTMENTS OR END BI	ENT SLOPES: NON	E			
INTERIOR BENTS:	SCOUR ON WEST BANK AT	NEW B2		***************************************	
	•				
CHANNEL BED: S	SCOUR POOL IN CHANNEL I	BED			
CHANNEL BANKS:S	SCOUR ON WEST BANK AND	O ALONG BAS	SE OF US321 E	MB. UPSTR. OF NEW	BRIDGE
<b>EXISTING SCOUR P</b>	ROTECTION:				
TYPE(3): NONE		-			
		WWF-III-planta and the communication			·
	I/A				
OBSTRUCTIONS(6) (DAN	MS,DEBRIS,ETC.): 3 BAF	RREL CULVER	RT AT MOUTH	OF TRIBUTARY 100 F	T UPSTR.
<b>DESIGN INFORMATI</b>	ON				
CHANNEL BED MATERIA	AL(7) (SAMPLE RESULTS AT	TACHED):	FINE TO CO	ARSE SAND AND GRA	VEL
<b>-</b>					
CHANNEL BANK MATER	IAL(8) (SAMPLE RESULTS A	TTACHED): _	SANDY SILT	(SE SS-1)	
FOUNDATION BEARING	MATERIAL(9): WEAT	THERED ROC	K AND HARD	ROCK	
CHANNEL BANK COVER	(10) GRASS	····		· · · · · · · · · · · · · · · · · · ·	
FLOOD PLAIN WIDTH(11	): 200 FEET				
FLOOD PLAIN COVER(12	2): GRASS AND FEW TREE	S			

DESIGN INFORMATION CONT. PAGE 2						
STREAM IS X DEGRADING AGGRADING (13)						
OTHER OBSERVATIONS AND COMMENTS: RECENT FLOOD ERODED BANK AT NEW B2-B SITE						
AND AT BASE OF US 321 EMBANKMENT						
CHANNEL MIGRATION TENDENCY (14): HARD CURRENT AGAINST WEST BANK AT NEW B2						
GEOTECHNICALLY ADJUSTED SCOUR ELEVATION (15):						
EBI-A 2626.3 FT EB1-B 2626.9 FT						
B1-A 2626.2 FT B1-B 2626.0 FT						
B2-A 2625.0 FT B2-B 2623.3 FT						
EB2-A 2625.3 FT EB2-B 2625.5 FT						
REPORTED BY: L. L. ACKER DATE: 6/9/04						
INSTRUCTIONS						
<ul> <li>GIVE THE DESCRIPTION OF THE SPECIFIC SITE GIVING ROUTE NUMBER AND BODY OF WATER CROSSED.</li> <li>NOTE ANY EVIDENCE OF SCOUR AT THE EXISTING END BENTS OR ABUTMENTS (UNDERMINING.</li> </ul>						
SLOUGHING, SCOUR LOCATIONS, DEGRADATIONS, ETC.)  (3) NOTE ANY EXISTING SCOUR PROTECTION (RIP RAP, ETC.)						

- (4) DESCRIBE THE EXTENT OF ANY EXISTING SCOUR PROTECTION.
- (5) DESCRIBE WHETHER OR NOT THE SCOUR PROTECTION APPEARS TO BE WORKING.
- 6) NOTE ANY DAMS, FALLEN TREES, DEBRIS AT BENTS, ETC.
- (7) DESCRIBE THE CHANNEL BED MATERIAL: A SAMPLE SHOULD BE TAKEN FOR GRAIN SIZE DISTRIBUTION, ATTACH LAB RESULTS.
- (8) DESCRIBE THE CHANNEL BANK MATERIAL: A SAMPLE SHOULD BE TAKEN FOR GRAIN SIZE DISTRIBUTION, ATTACH LAB RESULTS.
- 9) DESCRIBE THE FOUNDATION BEARING MATERIAL,
- (10) DESCRIBE THE BANK COVERING (GRASS, TREES, RIP RAP, NONE, ETC.
- (11) GIVE THE APPROXIMATE FLOOD PLAIN WIDTH (ESTIMATE).
- (12) DESCRIBE THE FLOOD PLAIN COVERING (GRASS, TREES, CROPS, ETC.)
- (13) CHECK THE APPROPRIATE SPACE AS TO WHETHER THE STREAM IS DEGRADING OR AGGRADING
- (14) DESCRIBE THE POTENTIAL OF THE BODY OF WATER TO MIGRATE LATERALLY DURING THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS).
- (15) GIVE THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION EXPECTED OVER THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS). THIS CAN BE GIVEN AS AN ELEVATION RANGE ACROSS THE SITE, OR ON A BENT BY BENT BASIS WHERE VARIATIONS EXIST. DISCUSS RELATIONSHIP BETWEEN THE HYDRAULICS THEORETICAL SCOUR AND THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION. THE GEOTECHNICALLY ADJUSTED SCOUR ELEVEVATION IS BASED ON THE ERODABILITY OF MATERIALS WITH CONSIDERATION FOR JOINTING, FOLIATION, BEDDING ORIENTATION AND FREQUENCY; CORE RECOVERY PERCENTAGE; PERCENTAGE ROD; DIFFERENTIAL WEATHERING, SHEAR STRENGTH; OBSERVATIONS AT EXISTING STRUCTURES; OTHER TESTS DEEMED APPROPRIATE; AND OVERALL GEOLOGIC CONDITIONS AT THE SITE.

JJL

M&T 503E

## NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT SOILS TEST REPORT-SOILS LABORATORY

T.I.P. ID #: 8-3922

REPORT ON SAMPLES OF: | Soil for Classification

PROJECT:	33356.1.1	COUNT	Y: Wa	tauga	***************************************	Owner:	No year	***************************************
DATE SAMPLED:	4-19-04	DATE RECE	EIVED:	5-3-04		DATE REI	ORTED:	5-20-04
SAMPLED FROM:	Rdw – Foundation SAMPLED BY:			ED BY:	LLAc	ker	**************************************	<u> </u>
SUBMITTED BY:	W D Frye	W D Frye			2002 STANDARD SPECIFICAT			CIFICATION
LABORATORY:	Asheville				-ll			

### TEST RESULTS

Project Sample No.	SS-1	SS-2	SS-3	SS-4	SS-5	SS-6	SS-7	T ·
Lab Sample No. A-	145275	145276	145277	145278	145279	145280	145281	
HiCAMS Sample #	***							
Retained #4 Sieve %								
Passing #10 Sieve %	99	88	100	73	100	100	100	
Passing #40 Sieve %	91	80	99	60	99	88	97	
Passing #200 Sieve %	42	59	77	41	89	60	64	<u> </u>

### MINUS #10 FRACTION

Soil Mortar - 100%								
Coarse Sand -Ret. #60	23	13	3	26	1	21	10	
Fine Sand - Ret. #270	48	34	34	26	23	33	39	
Silt 0.05-0.005 mm %	19	37	47	32	54	38	35	
Clay < 0.005 mm %	10	16	16	16	22	8	16	
Passing # 40 Sieve %								
Passing # 200 Sieve %								

Liquid Limit	27	29	38	31	51	32	34	
Plastic Index	NP	9	8	5	16	5	7	
AASHTO Classification	A-4 (1)	A-4 (5)	A-4 (8)	A-4 (1)	A-7-5 (13)	A-4 (5)	A-4 (6)	
Quantity								
Texture								
Station	10+79 Rt	10+28 Lt	10+28 Lt	10+21 Rt	10+21 Rt	10+121 Rt	11+85 Lt	
Hole No.	B1-B	EB1-A	EB1-A	EB1-B	EB1-B	EB1-A	EB2-A	
Depth (ft) From:	0.5	0.5	4.5	0.5	9.5	19.5	0.5	
To:	1.5	1.5	5.5	1.5	10.5	20.5	1.5	

### Remarks:

~	~	4

ce.	
W D Frye	
J J Lail	
File	

#### SOILS ENGINEER:

G:/Everyone. . . /M&T Forms/Regional Lab Statesville/Soils Test Report M&T 503E 8-

8-19-2000

PROJECT NO: 33356.1.1 (B-3922) WATAUGA COUNTY

### CORE PHOTOGRAPHS B1-B



B2-A



PROJECT NO: 33356.1.1 (B-3922) WATAUGA COUNTY



Figure 1. Bridge 316 on SR 1149. View looking upstream.



Figure 2. Looking downstream from Bridge 316 toward new bridge site on -L-. Willow tree is on Bent One. US 321 is on right side of photo.

PROJECT NO: 33356.1.1 (B-3922) WATAUGA COUNTY



Figure 3. Drilling on B1-A. View looking back along -L- from shoulder of US 321.

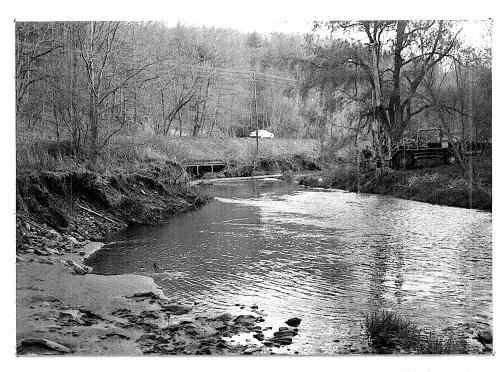


Figure 4. View looking upstream across -L-. Drilling on B1-A. Note fresh erosion on stream bank at B2 and at base of US 321 embankment.